- ALL DIMENSIONS ARE IN METRIC UNITS UNLESS NOTED. THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS AGAINST THE CIVIL, BUILDING, PROCESS, MECHANICAL, AND ELECTRICAL DRAWINGS THE EXISTING SITE CONDITIONS BEFORE BEGINNING CONSTRUCTION AND REPORT DISCREPANCIES TO THE ENGINEER BEFORE PROCEEDING WITH THE WORK. DO NOT SCALE DRAWINGS.
- THE DESIGN AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE NATIONAL BUILDING CODE OF CANADA 2020 WITH MANITOBA AMENDMENTS. IT'S SUPPLEMENTS AND THE LATEST EDITIONS OF REFERENCED CODES AND STANDARDS THEREIN, UNLESS NOTED OTHERWISE. BUILDING IMPORTANCE CATEGORY: POST-DISASTER.
- REFER TO THE BUILDING, PROCESS, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS AND DIMENSIONS OF OPENINGS, SLEEVES AND OTHER BUILDING COMPONENTS NOT SHOWN ON THE STRUCTURAL DRAWINGS. REPORT DISCREPANCIES AND OBTAIN ENGINEER'S PRIOR APPROVAL BEFORE INSTALLING SLEEVES AND OPENINGS THAT ARE NOT INDICATED ON THE STRUCTURAL DRAWINGS BEFORE PROCEEDING WITH CONSTRUCTION.
- REFER TO BUILDING, PROCESS, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATION OF PITS, BASES, DRAINS, TRENCHES, SUMPS, HOUSEKEEPING PADS, DEPRESSIONS, GROOVES, CURBS, CHAMFERS AND SLOPES NOT SHOWN ON THE STRUCTURAL DRAWINGS.
- CONTRACTOR TO CONFIRM WITH EQUIPMENT SUPPLIERS DIMENSIONS AND ALL OTHER CRITICAL DETAILS PRIOR TO CONSTRUCTION AND INSTALLATION. REPORT DISCREPANCIES AND OBTAIN APPROVAL PRIOR TO CONSTRUCTION.
- NOTIFY THE ENGINEER 48 HOURS IN ADVANCE FOR SITE REVIEW.
- DRAWINGS SHOW COMPLETED STRUCTURES ONLY. PROVIDE TEMPORARY BRACING FOR CONSTRUCTION LOADING CONDITIONS AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN LOADS SHOWN ON DRAWINGS.
- CONSTRUCTION METHODS REQUIRING TEMPORARY SHORING, OR BRACING, SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW. THE CONTRACTOR SHALL RETAIN A PROFESSIONAL ENGINEER, REGISTERED IN THE PROVINCE OF MANITOBA, TO PROVIDE ENGINEERING DESIGN AND TAKE RESPONSIBILITY FOR ANY SHORING AND BRACING OR OTHER WORK REQUIRING ENGINEERING DESIGNS TO COMPLETE THE CONSTRUCTION.
- VERIFY LOCATION OF UNDERGROUND SERVICES AND BE RESPONSIBLE FOR DISRUPTIONS.
- 12. ALL SHOP DRAWING SUBMITTALS TO BE METRIC (MILLIMETERS) UNLESS NOTED.

EXCAVATION & BACKFILL

- EXCAVATE TO LINES AND LEVELS NECESSARY TO PROPERLY COMPLETE THE WORK. CONTROL EXCAVATION TO ENSURE BOTTOM OF EXCAVATION DOES NOT SOFTEN DUE TO EXCESS MOISTURE. REFERENCE THE GEOTECHNICAL REPORT FOR FURTHER INFORMATION.
- ALL BACKFILL SHALL BE COMPACTED USING MECHANICAL EQUIPMENT. ON THE EXTERIOR OF THE STRUCTURES, THE BACKFILLING SHALL BE PLACED WITH SUFFICIENT ALLOWANCE FOR SETTLEMENT AND IN GENERAL, ITS TOP SURFACE SHALL BE NEATLY GRADED.
- MAINTAIN OPTIMUM MOISTURE CONTENT TO PERMIT COMPACTION TO ATTAIN SPECIFIED DENSITIES. PROTECT BACKFILLED GRADE, DURING AND AFTER COMPLETION OF BACKFILL OPERATION, FROM SOFTENING DUE TO EXCESS MOISTURE.
- BACKFILL TO GRADES INDICATED IN LAYERS NOT EXCEEDING 150mm UNCOMPACTED, UNLESS NOTED OTHERWISE.
- CONTRACTOR TO SUBMIT EXCAVATION SHOP DRAWING SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA.

DESIGN LOADS

- THE STRUCTURE IS DESIGNED TO MEET THE REQUIREMENTS OF THE 2023 MANITOBA BUILDING CODE.
- WIND LOADS THE BUILDING STRUCTURE IS DESIGNED TO RESIST THE HORIZONTAL LOADS RESULTING FROM A 1/50 AVERAGE HOURLY WIND PRESSURE BASED ON A q=0.45 kPa WITH AN IMPORTANCE CATEGORY OF "POST DISASTER".
- SNOW LOADS: THE ROOF AREA ARE DESIGNED BASED ON: GROUND SNOW LOAD = 1.9 kPa

RAIN LOAD = 0.2 kPa

- 4. FLOOR LOADS REFER TO STRUCTURAL DRAWINGS. SEISMIC LOADS: IMPORTANCE FACTOR:
- le = 1.5(ULS)SITE CLASSIFICATION: SPECTRAL ACCELERATION: Sa(0.2) = 0.133Sa(0.5) = 0.106
 - Sa(1.0) = 0.0548Sa(2.0) = 0.0215Sa(5.0) = 0.00432Sa(10.0) = 0.00126PEAK GROUND ACCELERATION: PGA = 0.0677
 - PEAK GROUND VELOCITY: PGV = 0.0542ACCELERATION SITE COEFFICIENT: Fa = 1.663VELOCITY SITE COEFFICIENT:
 - Fv = 2.673SEISMIC FORCE RESISTING SYSTEM: MASONRY SHEAR WALLS: Rd = 1.5

Ro = 1.5

MISCELLANEOUS METALS

- 1. THE STEEL ERECTOR SHALL BE RESPONSIBLE FOR SUPPLYING AND ERECTING ALL TEMPORARY WORKS REQUIRED FOR THE STRUCTURE DURING ERECTION.
- WELD TO CSA W59 BY FABRICATORS QUALIFIED TO CSA W47.1, IN DIVISION 2.
- ISOLATE MISC. METALS FROM FOLLOWING COMPONENTS BY MEANS OF 2 COATS OF AKALI RESISTANT BITUMINOUS PAINT: .1 DISSIMILAR METALS EXCEPT STAINLESS STEEL, GALVANIZED STEEL, ZINC, OR WHITE BRONZE OF SMALL AREA. .2 CONCRETE, MORTAR AND MASONRY.
- 4. STEEL PLATES: CONFORMING TO CSA G40.21; TYPE W WITH A MINIMUM YIELD STRENGTH OF 300 MPa.
- HP SHAPES: CONFORMING TO CSA G40.21; MINIMUM YIELD STRENGTH OF 350 MPa.
- 6. ANCHOR RODS: CONFORMING TO ASTM F1554.
- WELDING MATERIALS: CONFORMING TO CSA W59.

2. TEST CONCRETE IN ACCORDANCE WITH CSA A23.2.

- WELDING OF ALL LOAD CARRYING ASSEMBLIES IS TO BE PERFORMED BY A FIRM CERTIFIED BY THE CANADIAN WELDING BUREAU TO THE REQUIREMENTS OF
- SUPPLY ALL COMPONENTS REQUIRED FOR PROPER ANCHORAGE OF STEEL FABRICATIONS. FABRICATE ANCHORAGE AND RELATED COMPONENTS OF SAME MATERIAL AND FINISH AS STEEL FABRICATIONS, UNLESS OTHERWISE SPECIFIED OR SHOWN.
- 10. NELSON STUDS: H4L HEADED STUDS BY NELSON STUD, A DONCASTER GROUP LIMITED COMPANY OR APPROVED EQUAL.

FOUNDATION

CONCRETE

GEOTECHNICAL REPORT IS AVAILABLE AS REFERENCED IN THE CONTRACT SPECIFICATIONS.

CONTRACTOR SHALL HAVE A COPY OF THIS STANDARD ON SITE AT ALL TIMES.

PREVENT SUBGRADE FROM FREEZING AFTER CASTING CONCRETE WORKS UNTIL CONSTRUCTION IS COMPLETE AND STRUCTURES ARE IN

PROVIDE CONCRETE AND PERFORM WORK TO CSA A23.1. SUPPLY CONCRETE TO ALTERNATIVE (1) PERFORMANCE. THE

CONCRETE REINFORCEMENT

- DEFORMED BARS CONFORMING TO CSA G30.18 GRADE 400. LAP SPLICES SHALL BE CLASS B TENSION LAP TYPE AS NOTED IN THE BELOW TABLE, UNLESS NOTED
- WELDABLE REINFORCING BARS SHALL CONFORM TO CSA G30.18 GRADE 400W. WELDING OF REINFORCING SHALL CONFORM TO CSA W186.
- REINFORCING WORK SHALL BE IN ACCORDANCE WITH CSA A23.1 AND CSA A23.3.
- 4. REINFORCING STEEL SHALL BE DETAILED IN ACCORDANCE WITH THE LATEST EDITION OF THE REINFORCING STEEL INSTITUTE OF CANADA DETAILING MANUAL.
- 5. 90° HOOKS AND 180° HOOKS WHERE SHOWN SHALL BE DETAILED AS STANDARD HOOKS UNLESS NOTED OTHERWISE.
- 6. CONCRETE COVER TO REINFORCING STEEL SHALL CONFORM TO THE MOST STRINGENT REQUIREMENT LISTED BELOW UNLESS NOTED OTHERWISE.
- 7. SUBMIT SHOP DRAWINGS WHICH CLEARLY INDICATE BAR SIZES, GRADE, SPACING, HOOKS, BENDS AND SUPPORTING DEVICES, ETC. FOR REVIEW TO THE CONSULTANT ENGINEER PRIOR TO FABRICATION OF THE REINFORCING STEEL.

CONCRETE COVER					
SCUM BUILDING - ITEM	COVERAGE (mm)				
BOTTOM OF SLABS ON GRADE	75				
TOPS OF SLAB ON GRADE	50				
FORMED SURFACES NOT EXPOSED TO GROUND, WATER, OR NOT LISTED ABOVE	50				
HOUSEKEEPING PADS	50				
WALL, BEAM, COLUMN (OUTSIDE FACE OF TIES)	50				
PILE, PILE CAPES (OUTSIDE FACE OF TIES)	75				

	REINFORCING BAR DEVELOPMENT PER CSA 23.1								
	fy = 400 MPa								
	BAR SIZE	BAR DEVELOPMENT	COMPRESSION DEVELOPMENT	STANDARD HOOK DEVELOPMENT					
	10M	320mm	200mm	150mm					
	15M	480mm	280mm	180mm					
	20M	640mm	340mm	240mm					
	25M	990mm	440mm	300mm					

REINFORCING BAR SPLICES PER CSA 23.1						
f'c = 35 MPa fy = 400 MPa						
BAR SIZE	CLASS 'B' SPLICE					
10M	420mm					
15M	620mm					
20M	830mm					
25M	1290mm					

CONCRETE ACCESSORIES

- 1. GROUT: NON-SHRINK, NON-METALLIC GROUT WITH MINIMUM STRENGTH AT THREE DAYS OF 20 MPa AND MINIMUM STRENGTH AT 28 DAYS OF 50 MPa.
- EXPANSION ANCHORS: OF DIAMETER & PENETRATION SHOWN. CAPACITIES ARE BASED ON HILTI CANADA ANCHOR SYSTEMS. SUBMIT ANCHOR LOAD RESISTANCE DATA FROM INDEPENDENT TESTING FIRM FOR REVIEW BY CONTRACT ADMINISTRATOR MINIMUM 2 WEEKS PRIOR TO INTENDED USE.
- EPOXY ANCHORS: OF DIAMETER & PENETRATION SHOWN. SHEAR AND TENSION CAPACITIES ARE BASED ON HILTI HIT HY-200 + HIT-HAS SUPER HARDWARE. SUBMIT ANCHOR LOAD RESISTANCE DATA FROM INDEPENDENT TESTING FIRM FOR REVIEW BY ENGINEER MINIMUM 2 WEEKS PRIOR TO INTENDED USE.

CONCRETE MIX TYPES					
MIX TYPE	INTENDED APPLICATION	MINIMUM COMPRESSIVE STRENGTH (MPa)	CLASS OF EXPOSURE		
1	NON-STRUCTURAL CONCRETE FOR EXTERIOR WORKS - SLAB OR SLAB ON GRADE, BOLLARD INFILL	32 (28-DAY)	C-2		
2	STRUCTURAL CONCRETE FOR EXTERIOR WORKS	35 (56-DAY)	C-1		
3	STRUCTURAL OR NON-STRUCTURAL CONCRETE INTERIOR WORKS - SLAB, CURB, HOUSEKEEPING PADS, BEAMS	35 (28-DAY)	N		
4	LEAN MIX	15 (28-DAY)	N		
5	GROUT FOR CONCRETE USED IN MASONRY INFILL	20 (28-DAY)	N		
6	PILES, PILE CAPS, WALL, COLUMNS, GRADE BEAMS	35 (56-DAY)	S-1		
7	CONCRETE TOPPING	25 (28-DAY)	N		
8	NEW CONCRETE WITHIN EXISTING TREATMENT PLANT AREAS	35 (56-DAY)	A-1		

MASONRY

GROUT:

- MASONRY WORK SHALL BE IN ACCORDANCE WITH CSA S304.1 AND CSA A371.
- 2. CELLS CONTAINING REINFORCING SHALL BE COMPLETELY FILLED WITH GROUT IN LIFTS NOT EXCEEDING 2 METERS. CONSOLIDATE WITH GROUT BY VIBRATING DURING POURING.
- PROVIDE CLEAN-OUT HOLES IN THE BOTTOM COURSE OR ALL CELLS TO BE FILLED WITH GROUT. REMOVE ALL OVERHANGING MORTAR AND DEBRIS FROM INSIDE CELLS PRIOR TO GROUTING.
- 4. UNLESS INDICATED OTHERWISE, ALL MASONRY SHALL BE LAID IN RUNNING BOND. BOND ALL CORNERS AND INTERSECTIONS OF ALL LOAD BEARING WALLS, PROVIDE CONTROL JOINTS AT INTERSECTIONS OF NON-LOAD BEARING WALLS AND LOAD BEARING WALLS.
- PROVIDE CONTROL JOINT ON ONE SIDE OF ALL OPENINGS LESS THAN 2.0m IN WIDTH, ON BOTH SIDES OF OPENINGS LARGER THAN 2.0m IN WIDTH, MAXIMUM 3.0m FROM CORNERS AND MAXIMUM 6.0m o/c.
- MATERIALS SHALL CONFORM TO THE FOLLOWING:

CONCRETE MASONRY UNITS: CSA A165 SERIES, H/20/C/M 20 MPa MINIMUM COMPRESSIVE STRENGTH

MORTAR: CSA 179, TYPE S

(MINIMUM 28 DAY COMPRESSIVE STRENGTH OF MORTAR CUBES SHALL

BE 8.5 MPA IN ACCORDANCE WITH CSA A179 TABLE 6)

CSA 179, COARSE

(MINIMUM 28 DAY COMPRESSIVE STRENGTH OF GROUT CYLINDERS CAST IN NON-ABSORBENT MOULDS SHALL BE 12.5 MPa IN ACCORDANCE

WITH CSA A179 TABLE 7)

CSA G30.18. GRADE 400. PLAIN FINISH, DEFORMED BARS REINFORCING STEEL:

ALIGNMENT UNTIL ALL STRUCTURAL WORK AND CONNECTIONS HAVE BEEN COMPLETED.

JOINT REINFORCING: ASTM A370, CONTINUOUS WELDED LADDER REINFORCING CONTRACTOR SHALL PROVIDE ALL REQUIRED TEMPORARY WALL BRACING DURING REQUIRED TO ERECT AND HOLD THE MASONRY WALLS IN PROPER

- HEAT MATERIALS AND PROTECT WORK IN ACCORDANCE WITH CSA-S304 WHEN AMBIENT TEMPERATURE IS BELOW 5° C.
- PROVIDE SOLID BLOCK OR CONCRETE FILLED BLOCK UNDER ALL CONCENTRATED LOADS BEARING ON MASONRY.
- 10. PROVIDE CONCRETE FILLED CORES AT ALL LOCATIONS WHERE METAL FABRICATIONS OR OTHER EQUIPMENT, UTILITIES, ETC., ARE FASTENED TO BLOCK
- 11. PROVIDE DOWELS FROM CONCRETE BEAMS, WALL AND FLOORS TO MATCH MASONRY WALL REINFORCING UNLESS NOTED OTHERWISE.
- 12. PROVIDE BOND BEAMS AT TOP OF WALL AND EVERY 2400mm (MAX) VERTICAL SPACING UNLESS NOTED OTHERWISE.
- 13. TRUSS TYPE JOINT REINFORCING AT EVERY SECOND COURSE.

ALUMINUM CONNECTION DISCLAIMER

ALL ALUMINUM CONNECTIONS SHOWN ARE FOR DIAGRAMMATIC PURPOSES ONLY. ALUMINUM FABRICATOR MUST DESIGN CONNECTIONS TO SUIT LOADS SHOWN ON DRAWINGS.

STEEL CONNECTION DISCLAIMER

ALL STEEL CONNECTIONS SHOWN ARE FOR DIAGRAMMATIC

PURPOSES ONLY. STEEL FABRICATOR MUST DESIGN CONNECTIONS TO SUIT LOADS SHOWN ON DRAWINGS.

ENGINEERS GEOSCIENTISTS MANITOBA **Certificate of Authorization AECOM Canada ULC** No. 4671

AECOM DESIGNED BY: DRAWN BY: APPROVED BY: SCALE: As indicated RELEASED FOR CONSTRUCTION 2023-07-24 00 ISSUED FOR TENDER 2025-02-26 | CT | MB | CONSULTANT NO.: U2-S001 NO. REVISIONS DATE DESIGN CHECK

BROTHERSTON Member 25515

TENDER NO. 30-2025 THE CITY OF WINNIPEG

NORTH END SEWAGE TREATMENT PLANT NEWPCC UV UPGRADES STRUCTURAL NEW UV STORAGE ROOM

GENERAL NOTES CITY DRAWING NUMBER

Winnipeg

SHEET | REV. | SIZE

WATER AND WASTE DEPARTMENT

- 3. ALL HOLLOW STRUCTURAL STEEL SECTIONS SHALL CONFORM TO CSA G40.21, GRADE 350W CLASS C
- ALL WELDING SHALL CONFORM TO CSA S16 AND LATEST VERSION OF W59 AND SHALL BE PERFORMED BY A WELDER QUALIFIED UNDER THE LATEST VERSION OF
- ALL SHOP CONNECTIONS SHALL BE WELDED. ALL FIELD CONNECTIONS SHALL BE WELDED OR BOLTED USING HIGH TENSILE BOLTS. BEARING TYPE CONNECTIONS SHALL BE C.I.S.C DOUBLE ANGLE BEAM CONNECTIONS OR SHEAR PLATES USING A325 BOLTS AND E49XX FILLET WELDS, MINIMUM SIZE OF BOLTS -20MM DIAMETER.
- THE CONNECTIONS SHALL BE CAPABLE OF SUPPORTING 50% OF THE TOTAL UNIFORMLY DISTRIBUTED FACTORED LOAD FOR BEAMS LATERALLY SUPPORTED EXCEPT WHERE SPECIFICALLY NOTED OR DETAILED, IN ADDITION TO THE TRANSFER OF FACTORED MOMENTS, WHERE SHOWN ON HE DRAWINGS.
- 7. SPLICING OF MEMBERS NOT PERMITTED UNLESS OTHERWISE NOTED.
- BRACING CONNECTIONS SHALL BE DESIGNED FOR 50% OF MEMBER TENSILE STRENGTH UNLESS MEMBER FORCE IS INDICATED ON DRAWINGS.
- 9. ALL BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO (2) A325 BOLTS IN EACH CONNECTED PIECE AND BE DESIGNED WITH BEARING TYPE CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLANE, UNLESS NOTED OTHERWISE.
- 10. PROVIDE AND TAKE RESPONSIBILITY FOR ALL TEMPORARY BRACING AND SHORING REQUIRED TO PROVIDE STABILITY FOR THE STRUCTURE AS A HOLE. THESE SHALL REMAIN IN PLACE UNTIL FLOOR SLABS ARE WELL CURED, STEEL ROOF DECK IS FULLY WELDED AND/OR PERMANENT RACING IS INSTALLED.
- 11. WELDING ELECTRODES SHALL BE E49XX
- 12. SURFACES TO BE WELDED SHALL BE THOROUGHLY CLEANED OF ALL FOREIGN MATTER INCLUDING PAINT FILM.
- 13. ALL HSS SECTIONS MUST HAVE OPEN ENDS CAPPED OR WELDED SOLID ALL AROUND AT CONNECTION POINT.
- 14. ALL COLUMN ENDS SHALL BE SAWCUT AND WELDED TO BASE PLATES.
- 15. PROVIDE WELDED STIFFENER PLATES ON BOTH SIDES, UNLESS NOTED, OF THE WEB OF BEAMS AT POINTS OF CONCENTRATED LOAD INCLUDING BEAMS SUPPORTING COLUMNS OR RUNNING OVER TOP OF COLUMNS. MINIMUM STIFFENER PLATE THICKNESS SHALL BE 10MM OR FLANGE THICKNESS OF COLUMNS ABOVE OR BELOW, WHICHEVER IS GREATER. MINIMUM SIZE OF WELD SHALL BE 5MM DOUBLE FILLET WELD, OR SHALL BE SUFFICIENT TO DEVELOP THE FULL STRENGTH OF THE STIFFENER, WHICHEVER IS GREATER.
- 16. WHERE BEAMS ARE CONTINUOUS OVER SUPPORTS, NO HOLES PERMITTED IN TOP FLANGE. PROVIDE 2-10MM THICK WELDED WEB STIFFENER PLATE'S (OR SIZE AS DESCRIBED ABOVE) EACH SIDE OF BEAM, ALIGNED WITH COLUMN WALLS.
- 17. ALL COLUMNS TO HAVE CLOSURE PLATES, TEES ANGLES OR OUTRIGGERS AT FLOOR AND ROOF LEVELS TO SUPPORT STEEL DECK WHERE REQUIRED AND TO PREVENT CONCRETE LOSS (ELEVATED FLOORS).
- 19. GUSSET PLATES FOR DIAGONAL BRACING SHALL BE CONNECTED TO ALL INTERSECTING MEMBERS UNLESS NOTED OTHERWISE, AND BE IN LINE WITH CENTRELINE OF MEMBERS.
- 20. IN ADDITION TO STRENGTH WELDS, STRUCTURAL STEEL EXPOSED TO WEATHER SHALL HAVE CONTINUOUS SEAL WELDS AT ALL JOINTS INCLUDING ALL
- 21. THERE SHALL BE NO CUTTING OF STRUCTURAL STEEL MEMBERS FOR THE WORK OF OTHER TRADES WITHOUT PRIOR WRITTEN APPROVAL OF THE PROFESSIONAL
- 22. FOR PRIME PAINTING / PAINTING OF STRUCTURAL STEEL, REFER TO SPECS. TOUCH-UP DAMAGED AREAS IN FIELD WITH SAME SYSTEM.
- 23. SUBMIT SHOP DRAWINGS BEARING THE SEAL OF A PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA SHOWING ALL DESIGN AND FABRICATION DETAILS OF CONNECTIONS FOR REVIEW PRIOR TO FABRICATION.

OPEN WEB STEEL JOISTS

- 1. DESIGN, FABRICATE AND INSTALL OPEN WEB STEEL JOISTS IN ACCORDANCE WITH CSA S16 AND THE CISC CODE OF STANDARD PRACTICE FOR STRUCTURAL STEEL.
- WELDING SHALL BE IN ACCORDANCE WITH CSA W59. FABRICATOR TO BE CERTIFIED UNDER DIVISION 1 OR 2.1 OF CSA W47.1 FOR FUSION WELDING OF STEEL STRUCTURES, AND/OR CSA W55.3 FOR RESISTANCE WELDING OF STRUCTURAL COMPONENTS.
- COORDINATE BRIDGING TYPE AND LOCATIONS WITH THE CONTRACTOR TO ENSURE FITMENT OF ALL DUCT AND PIPE RUNS, AND OTHER SERVICES PASSING THROUGH THE JOISTS.
- PROVIDE TEMPORARY BRACING AS REQUIRED DURING CONSTRUCTION.
- DESIGN JOISTS, BRIDGING, AND ANCHORAGES IN ACCORDANCE WITH CSA S16 FOR THE DESIGN LOADS LISTED ON PLAN.
- 7. LIMIT JOIST DEFLECTIONS DUE TO SPECIFIED LIVE LOADS TO L/600.
- DESIGN, DETAIL AND FABRICATE BEARING SHOES TO SEAT AT CENTRE OF STEEL SUPPORTS EXCEPT WHERE JOISTS ARE FROM ONE SIDE IN WHICH CASE THE BEARING SHOE SHALL BE WITHIN THE MIDDLE THIRD OF THE BEAM FLANGE.
- DESIGN AND DETAIL CONNECTIONS AND BEARINGS NOT SHOWN ON THE DRAWINGS FOR FORCES SHOWN ON THE PLANS.
- 10. DESIGN AND PROVIDE BOTTOM CHORD EXTENSION FOR FASTENING TO COLUMNS AS NECESSARY FOR ERECTION AND WHERE NOTED.
- 11. HANDLE JOISTS WITH CARE TO AVOID DAMAGE. DO NOT USE JOISTS WITH DEFORMED MEMBERS.
- 12. SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. SHOP DRAWINGS SHALL SHOW LAYOUT, MARKING AND SPACING DETAILS, MATERIALS, UNIFORM AND CONCENTRATED DESIGN LOADS, BRIDGING AND ACCESSORIES. SHOP DRAWINGS SHALL BE SEALED BY A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE
- 13. THE CONTRACTOR'S PROFESSIONAL ENGINEER SHALL CARRY OUT SITE REVIEW OF INSTALLED COMPONENTS AND ISSUE A LETTER OF SUPERVISION STATING THAT THE WORK COMPLIES WITH THE CONTRACT REQUIREMENTS.

METAL DECKING

- METAL DECKING WORK SHALL BE IN ACCORDANCE WITH CSA S136 AND CSSBI 10M.
- DESIGN STEEL DECK TO:
- A. SUSTAIN WORST FACTORED LOADING COMBINATION.
- B. LIMIT VERTICAL DEFLECTION TO MEET CRITERIA AS PER TABLE D1 OF CAN/CSA S16.
- C. ACT AS FLOOR OR ROOF DIAPHRAGM WITH MINIMUM SHEAR STRENGTH AND STIFFNESS AS NOTED ON THE PLANS.
- D. SUSTAIN CONSTRUCTION LOADS INCLUDING WEIGHT OF WET CONCRETE WITHOUT UNDUE DEFLECTION.
- WELDING SHALL BE IN ACCORDANCE WITH CSA W59. FABRICATOR TO BE CERTIFIED UNDER DIVISION 1 OR 2.1 OF CSA W47.1 FOR FUSION WELDING OF STEEL STRUCTURES, AND/OR CSA W55.3 FOR RESISTANCE WELDING OF STRUCTURAL COMPONENTS.
- 4. REINFORCE DECK OPENINGS SMALLER THAN 450 mm IN LENGTH AND WIDTH AS RECOMMENDED BY DECK SUPPLIER.
- MATERIALS SHALL CONFORM TO THE FOLLOWING:
- ZINC-IRON ALLOY (Z) COATED STEEL SHEET: ASTM A653M, STRUCTURAL QUALITY GRADE 230, WITH Z275 COATING.
- METAL DECKING SHALL CONFORM TO THE FOLLOWING: ROOF DECK:
- 38 mm DEEP PROFILE, MINIMUM 0.91 mm BASE STEEL THICKNESS, ACOUSTICAL DECK. CANAM P3615 OR APPROVED EQUIVALENT. MINIMUM 3 SPANS CONTINUOUS
- 7. CONNECTIONS SHALL BE IN ACCORDANCE WITH CSSBI RECOMMENDATIONS:
- ALL SUPPORTING STEEL: 19 mm PUDDLE WELDS AT 300 o/c (36/4 SUPPORT PATTERN)
- AROUND PERIMETER: 19 mm PUDDLE WELDS AT 300 o/c SIDE LAPS: BUTTON PUNCH AT 600 mm o/c
- 8. INSTALL DECKING IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS:
 - A. DECKING CONTINUOUS OVER MINIMUM THREE SPANS WHERE VER STRUCTURAL FRAMING PERMITS.
- ENSURE MINIMUM BEARING EQUAL TO DECK DEPTH, LAP JOINTS 6 MM AT STRUCTURAL SUPPORTS.
- C. INSTALL CLOSURES AND FLASHINGS AT SLAB EDGES, WALLS, COLUMNS AND OPENINGS FOR COMPOSITE DECK.
- SUBMIT SHOP DRAWINGS SHOWING ALL DETAILS, MATERIAL SPECIFICATIONS AND DESIGN LOADS. DETAILS TO INCLUDE ANCHORAGE DETAIL, REINFORCING TO OPENINGS, ACCESSORIES AND ATTACHMENTS. SHOP DRAWINGS SHALL BE SEALED BY A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF MANITOBA.

STRUCTURAL STANDARD ABBREVIATIONS:

ACOUSTIC	ACST	HANGER	HGR
ADDITIONAL ADHESIVE	ADD`L ADH	HEIGHT HEXAGON	HT HEX
ADJUSTABLE	ADJ	HOLLOW STRUCTURAL STEEL	HSS
AGGREGATE	AGGR	HORIZONTAL	HORIZ
AIR HANDLING UNIT AIR VAPOUR BARRIER	A.H.U. A.V.B.	INCLUSIVE INSIDE DIAMETER	INCL I.D.
ALTERNATE	ALTER	INSIDE FACE	I.F.
ALUMINUM ANCHOR BOLT	ALUM A. BOLT	INTERIOR INVERT ELEVATION	INT INVT EL
ANCHOR BOLT APARTMENT	APT	JUNCTION	JCT
APPROXIMATE	APPROX	KILONEWTON	kN
BUILDING AT	BUILDING @	KNOCK DOWN LIVE LOAD	K.D. L.L.
AVERAGE	AVG	LONG	LG.
BEARING BENGLI MARK	BRG		L.L.H.
BENCH MARK BETWEEN	B.M. BET	LONG LEG VERTICAL LOUVER	L.L.V. LVR
BOARD	BD	MAKE UP AIR UNIT	MAU
BOTH SIDES BOTH WAYS	B.S. B.W.	MANUFACTURE MARK	MFG MK
BOTTOM	BOT	MASONRY OPENING	M.O.
BOTTOM LOWER LAYER	BLL	MATERIAL	MATL
BOTTOM UPPER LAYER BUILDING	BUL BLDG	MAXIMUM MECHANICAL	MAX MECH
BY (BETWEEN DIMS)	x (lower case)	METER	m (lower case)
CAST IN PLACE CAST IRON	C.I.P. C.I.	MILLIMETER MINIMUM	mm (lower case) MIN
CATCH BASIN	0 B	MINIMUM MISCELLANEOUS	MISC
CEMENT BOARD	C.B. C.BD.	NEAR FACE	N.F.
CENTERLINE CENTER TO CENTER	C C/C	NEAR SIDE NORTH	N.S. N
CIRCULAR	CIRC	NOT APPLICABLE	N/A
CLEAN OUT	C.O.	NOT IN CONTRACT	N.I.C.
CLEAR SPAN COLUMN	CL. SPAN COL	NOT TO SCALE NUMBER	N.T.S. No.
COMPLETE WITH	c/w	ON CENTER	o/c (lower case)
CONCRETE CONCRETE MASONRY UNIT	CONC CMU	OPENING OPEN WEB STEEL JOIST	OPG O.W.S.J.
CONCRETE PIPE	C.P.	OPPOSITE	O.W.S.J.
CONSTRUCTION	CONSTR	OPPOSITE ORIGINAL OUTSIDE DIAMETER	ORIG
CONSTRUCTION JOINT CONTINUOUS	C.J. CONT	OUTSIDE DIAMETER	O.D. O.F.
CORRUGATED METAL PIPE	C.M.P.	OUTSIDE FACE OUT TO OUT OVERHEAD PERPENDICULAR	0/0
COUNTERSUNK	CSK	OVERHEAD	O/H
DEAD LOAD DEGREE	D.L.	PERPENDICULAR PLATE	PERP PL
DIAMETER	DIA	POLINDS PER LINEAR FOOT	PLF
DIMENSION	DIM DN	POUNDS PER SQUARE FOOT PLYWOOD PRE-CAST PREFABRICATED PREINISHED PRELIMINARY PROJECTION QUANTITY RADIUS REFERENCE REINFORCE WITH REINFORCING REQUIRED RETAINING WALL REVISION ROOF DRAIN ROUGH OPENING SCHEDULE SECTION SELF ADHESIVE SHEATHING SHEET SIMILAR SKETCH SOUTH SPECIFICATION SQUARE FEET STAINLESS STEEL STANDARD STIFFENER STIRRUP STRUCTURAL	PSF PLYWD
DIMENSION DOWN DOWEL DRAWING EACH FACE EACH WAY EAST ELECTRICAL	DWL	PRE-CAST	P/C
DRAWING	DWG	PREFABRICATED	PREFAB
EACH FACE FACH WAY	E.F. E.W.	PREFINISHED PREFIMINARY	PREFIN PRELIM
EAST	E	PROJECTION	PROJ
ELECTRICAL	ELECT	QUANTITY	QTY
ELEVATION FLEVATOR	EL ELEV	REFERENCE	R REF
ELECTRICAL ELEVATION ELEVATOR EQUAL EQUAL SPACE EXCAVATION EXISTING EXPANSION	EQ.	REINFORCE WITH	R/W
EQUAL SPACE	EQ SP EXC	REINFORCING	REINF REQ`D
EXISTING	EXIST EXP	RETAINING WALL	R.W.
EXPANSION	EXP E.J.	REVISION	REV
EXPANSION JOINT EXTERIOR	E.J. EXT	ROUGH OPENING	R.D. R.O.
FACE TO FACE	F/F	SCHEDULE	SCHED
FACE OF CONCRETE FAR SIDE	F.O.C. F.S.	SECTION SELE ADHESIVE	SECT S.A.
	FBRBD	SHEATHING	SHTG
FIBREBOARD FINISH FLOOR DRAIN	FIN	SHEET	SHT
FLOOR DRAIN FOUNDATION	F.D. FDN	SIMILAR SKETCH	SIM SK
FOUNDATION FOOTING	FTG	SOUTH	S
GALVANIZED IDON	GALV G.I.	SPECIFICATION SOLVABLE FEET	SPEC SQ. FT.
GALVANIZED IRON GAUGE	G.I. GA	STAINLESS STEEL	S.S.
GRANULAR	GRAN	STANDARD	STD
GRANULAR BASE GRANULAR BACK FILL	G.B. GBFL	STIFFENER STIRRUP	STIFF STIRR
GRID LINE	G.L.	STRUCTURAL	STRUCT
GUARD RAIL	G.R.	SYMMETRICAL	SYM
		TANGENT TEMPORARY	TAN TEMP
		TO MATCH EXISTING	T.M.E.
		TOP OF TOP LOWER LEVEL	T.O. TLL
		TOP LOWER LEVEL TOP UPPER LEVEL	TUL
		TYPICAL	TYP
		UNDERCUT UNLESS NOTED	U. CUT U/N
		VERTICAL	VERT
		WEIGHT	WT
		WIND LOAD WITH	W.L. w/
		WITH OUT	W/O
		WOOD WROUGHT IRON	WD W.I.
		WINDOOM INON	¥¥.1.

ENGINEERS GEOSCIENTISTS MANITOBA **Certificate of Authorization AECOM Canada ULC** No. 4671

AECOM DESIGNED BY: CK DRAWN BY: APPROVED BY: SCALE: As indicated RELEASED FOR CONSTRUCTION 2023-07-24 00 ISSUED FOR TENDER 2025-02-26 | CT | MB [CONSULTANT NO.: U2-S002 DATE DESIGN CHECK NO. REVISIONS

BROTHERSTON Member 25515 2025-02-27

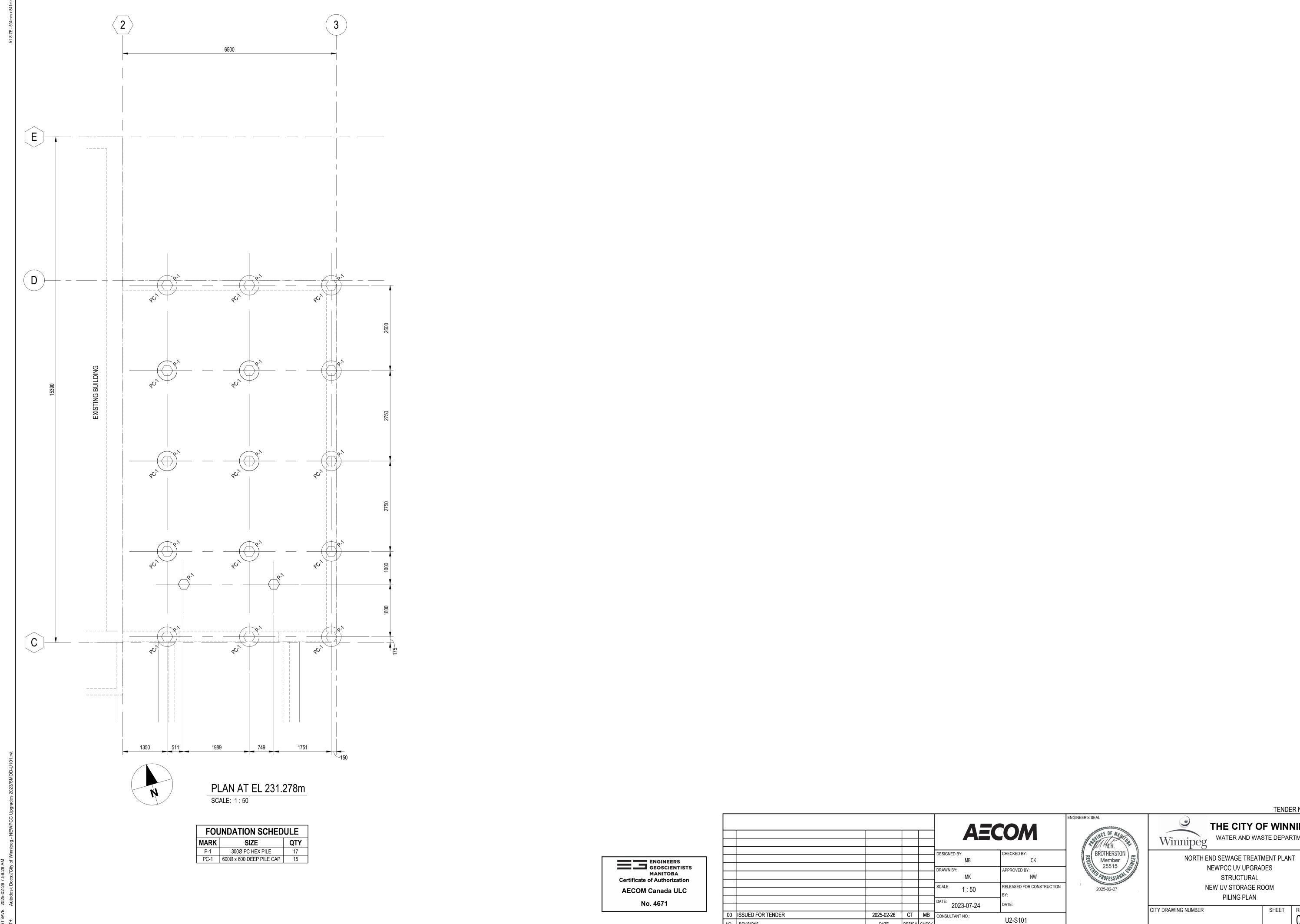
THE CITY OF WINNIPEG WATER AND WASTE DEPARTMENT Winnipeg

> NORTH END SEWAGE TREATMENT PLANT NEWPCC UV UPGRADES STRUCTURAL NEW UV STORAGE ROOM

GENERAL NOTES CITY DRAWING NUMBER

SHEET | REV. | SIZE

TENDER NO. 30-2025



NO. REVISIONS

DATE DESIGN CHECK

TENDER NO. 30-2025

SHEET REV. SIZE 00 A1

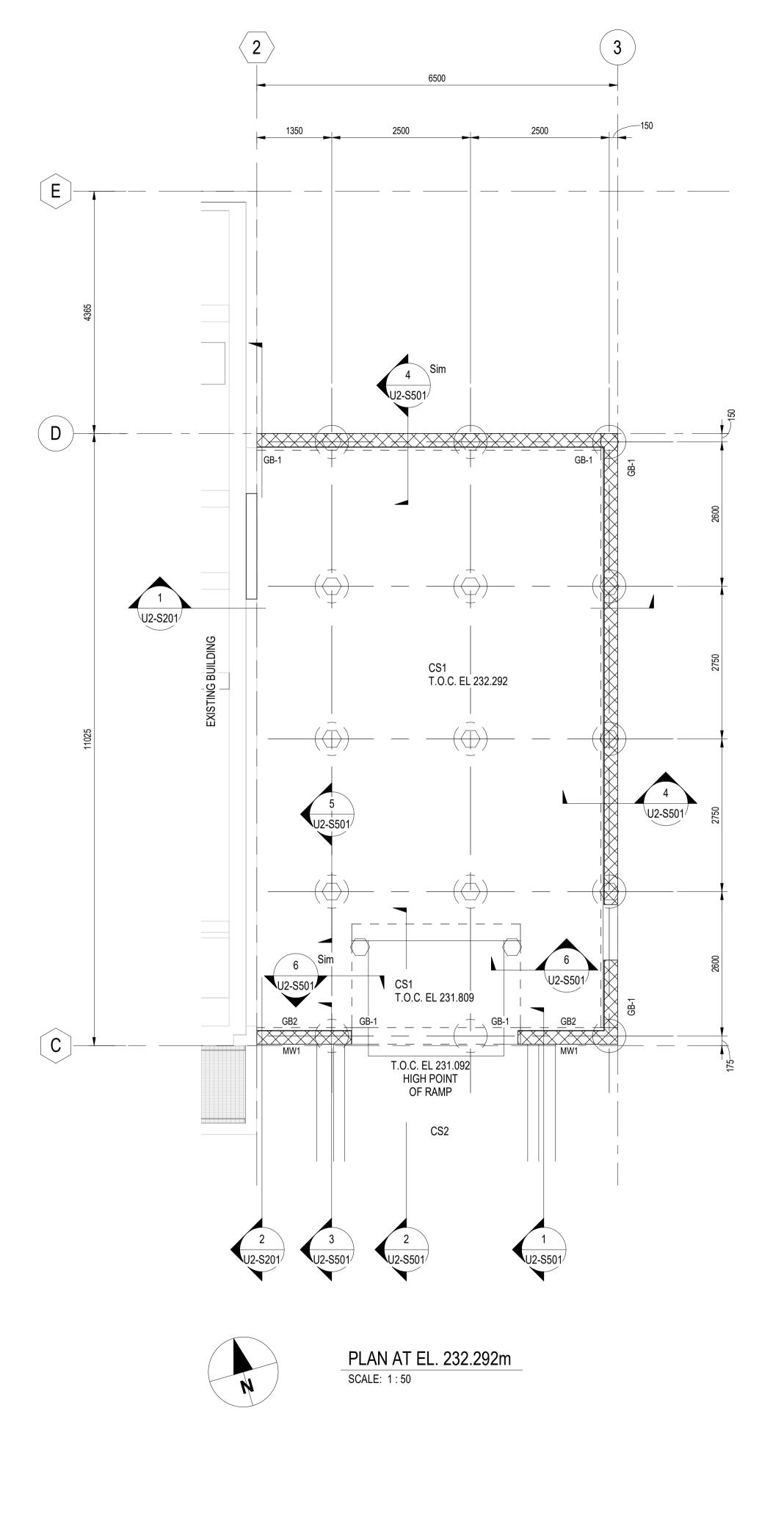
THE CITY OF WINNIPEG

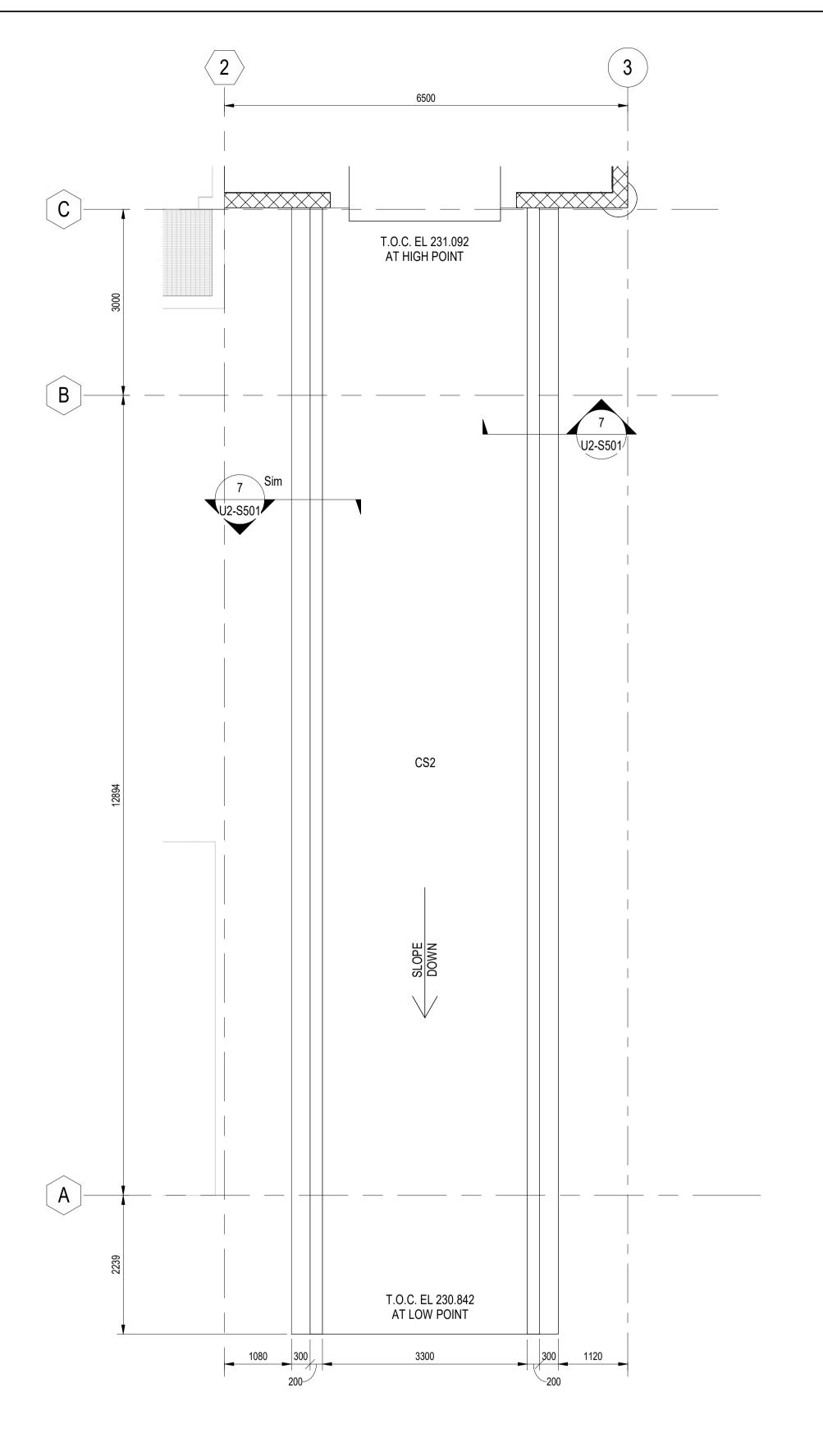
WATER AND WASTE DEPARTMENT

NEWPCC UV UPGRADES

STRUCTURAL

PILING PLAN





PLAN AT EL 232.292m SCALE: 1:50

NO. REVISIONS

ENGINEERS GEOSCIENTISTS

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AECOM Canada ULC

No. 4671

MANITOBA

AECOM APPROVED BY: SCALE: As indicated RELEASED FOR CONSTRUCTION 2023-07-24 00 ISSUED FOR TENDER 2025-02-26 CT MB CONSULTANT NO.: U2-S102

DATE DESIGN CHECK

2025-02-27

TENDER NO. 30-2025 THE CITY OF WINNIPEG Winnipeg WATER AND WASTE DEPARTMENT

NORTH END SEWAGE TREATMENT PLANT NEWPCC UV UPGRADES STRUCTURAL NEW UV STORAGE ROOM MAIN FLOOR PLAN

CITY DRAWING NUMBER

SHEET REV. SIZE 00 A1

SLAB REINFORCING SCHEDULE
CS1: 300 SLAB r/w 15M AT 300oc EW T & B

L = 4.8 kPa

1. REFER TO ELECTRICAL, MECHANICAL AND PROCESS DRAWINGS FOR SIZE AND LOCATIONS

STRUCTURE SELF WEIGHT

OF PENETRATIONS THROUGH THE FLOOR. SLEEVE ALL PENETRATIONS.
REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS OF INTERIOR WALLS.

BUL & TLL BLL & TUL

CS2: 200 SLAB r/w 15M AT 200oc EW MID BLL

- 5. ALL SLAB REINFORCING TO BE CONTINUOUS, PROVIDE CLASS B TENSION SPLICES WHERE REQUIRED FOR LENGTH.
- 6. PROVIDE STANDARD HOOKS FOR ALL TOP REINFORCING AT DISCONTINUOUS EDGES.

GRADE BEAM SCHEDULE:

NOTES:

DEAD LOADS:

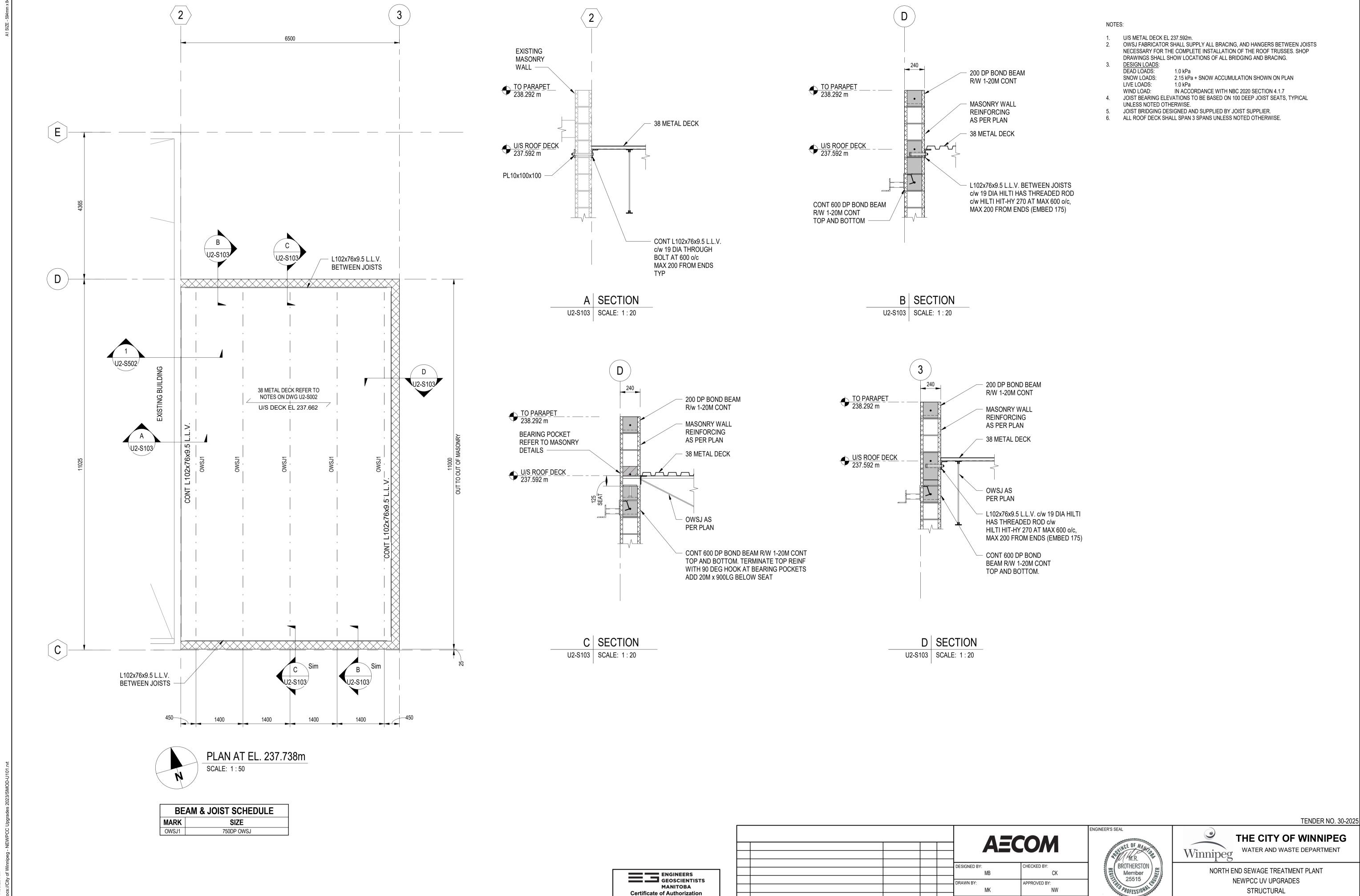
LIVE LOADS:

- GB-1: 300x1200DP GRADE BEAM R/W 3-25M CONT T&B C/W 15M HEF AND 10M CLOSED STIRRUPS AT 300 o/c
- GB-2: 300x1400DP GRADE BEAM R/W 3-25M CONT T&B C/W 15M

HEF 10M CLOSED STIRRUPS AT 300 o/c

MASONRY WALL REINFORCING:

- A. COREFILL AND REINFORCE EXTERIOR 240mm WALLS WITH 1-15M VERT FULL HEIGHT AT MAX
- PROVIDE A 600 DP CONT BOND BEAM AND REINF WITH 1-20M TOP AND BOTTOM CONT AT ROOF DECK ELEVATION. SEE DETAILS ON DRAWING U2-S-103.
- PROVIDE A 200 DP BOND BEAM REINF WITH 1-20M CONT AT TOP OF PARAPET.
- INSTALL HORIZ JOINT REINFORCING AT 400 o/c. LAP REINF A MIN OF 2 CROSS WIRE
- E. PROVIDE REINFORCED LINTELS WITH ABOVE ALL OPENINGS, REFER TO DRAWING U2-S502.



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CITY DRAWING NUMBER SHEET REV. SIZE 00 A1

NEW UV STORAGE ROOM

ROOF FRAMING PLAN

SCALE: As indicated

CONSULTANT NO.:

2025-02-26 | CT | MB

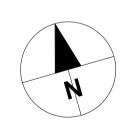
DATE DESIGN CHECK

2023-07-24

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U2-S103

2025-02-27

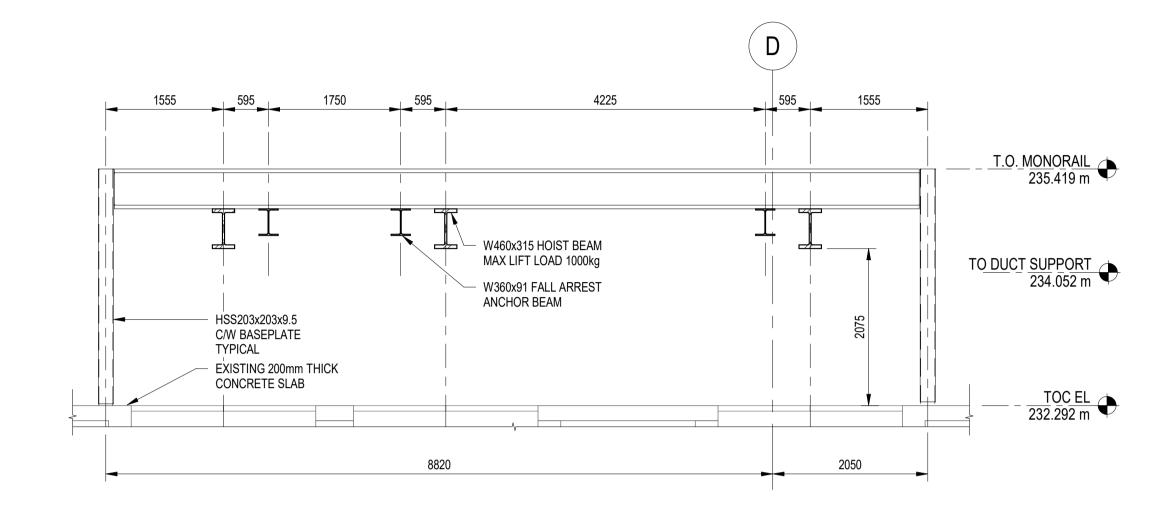


MONORAIL FRAMING PLAN

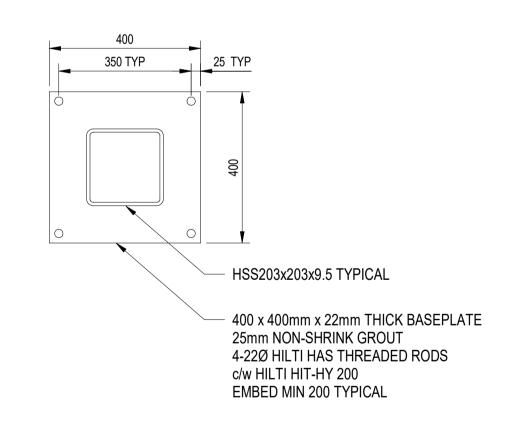
SCALE: 1:75

NOTE:

- 1. WHERE MOMENT CONNECTIONS ARE SPECIFIED BUT VALUE IS NOT SHOWN, DESIGN FOR FULL MOMENT CAPACITY OF THE SMALLER
- MEMBER JOINED. 2. SITE VERIFY MONORAIL FRAMING DIMENSIONS



1 SECTION U2-S104 SCALE: 1:50



BASEPLATE DETAIL SCALE: 1:10

ENGINEERS GEOSCIENTISTS **MANITOBA** Certificate of Authorization **AECOM Canada ULC** No. 4671

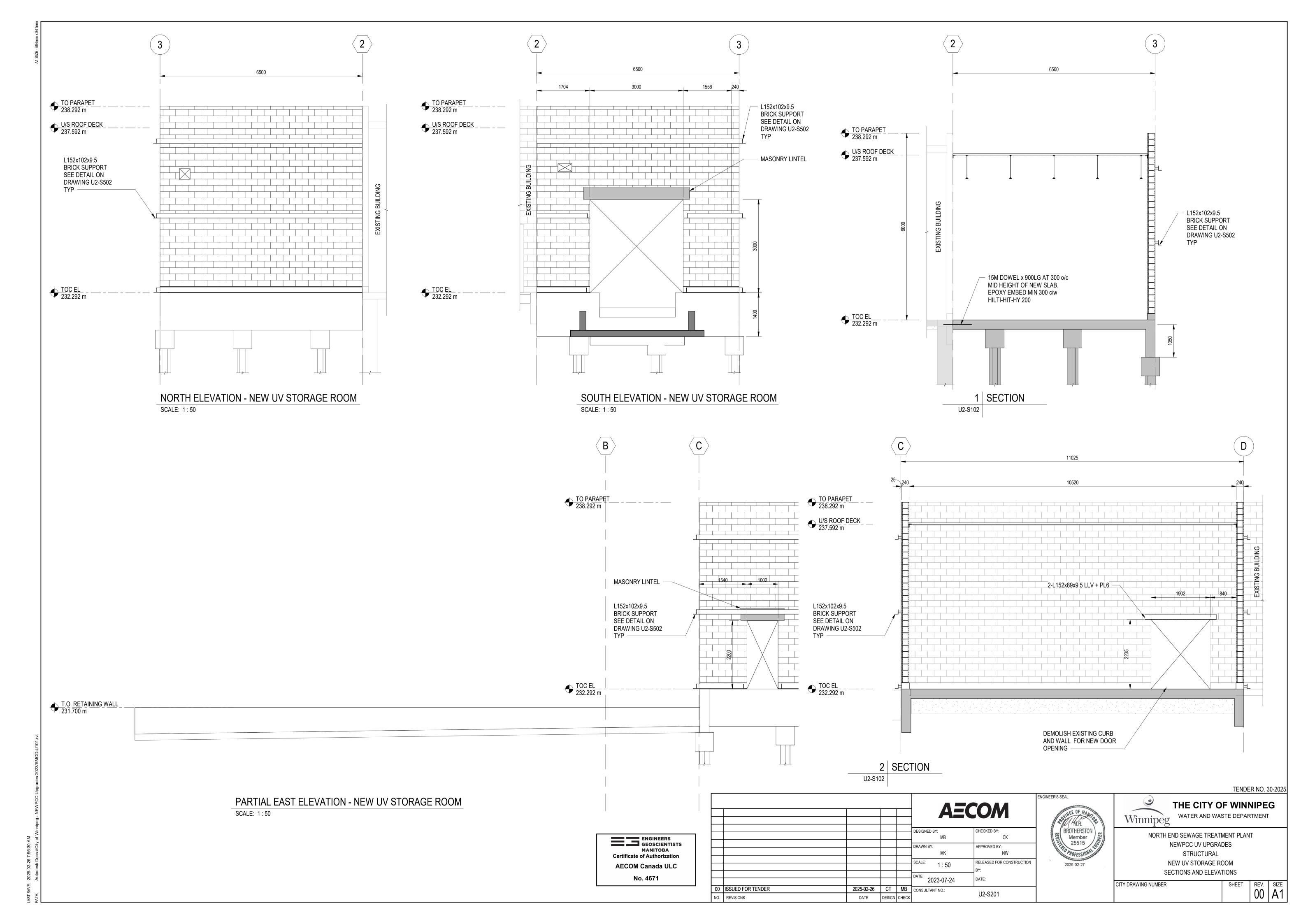
						A =/	2014	ENGINEER'S SEAL
						AEC	MO	RELINCE OF M
1						DESIGNED BY:	CHECKED BY: MB	1 # // pporturne
						DRAWN BY: BF	APPROVED BY:	Member 25515
						SCALE: As indicated	RELEASED FOR CONSTRUCTION BY:	2025-02-27
						DATE: 2023-07-24	DATE:	
	00	ISSUED FOR TENDER	2025-02-26	СТ	MB	CONSULTANT NO.:	U2-S104]
	NO.	REVISIONS	DATE	DESIGN	CHECK		02-3104	

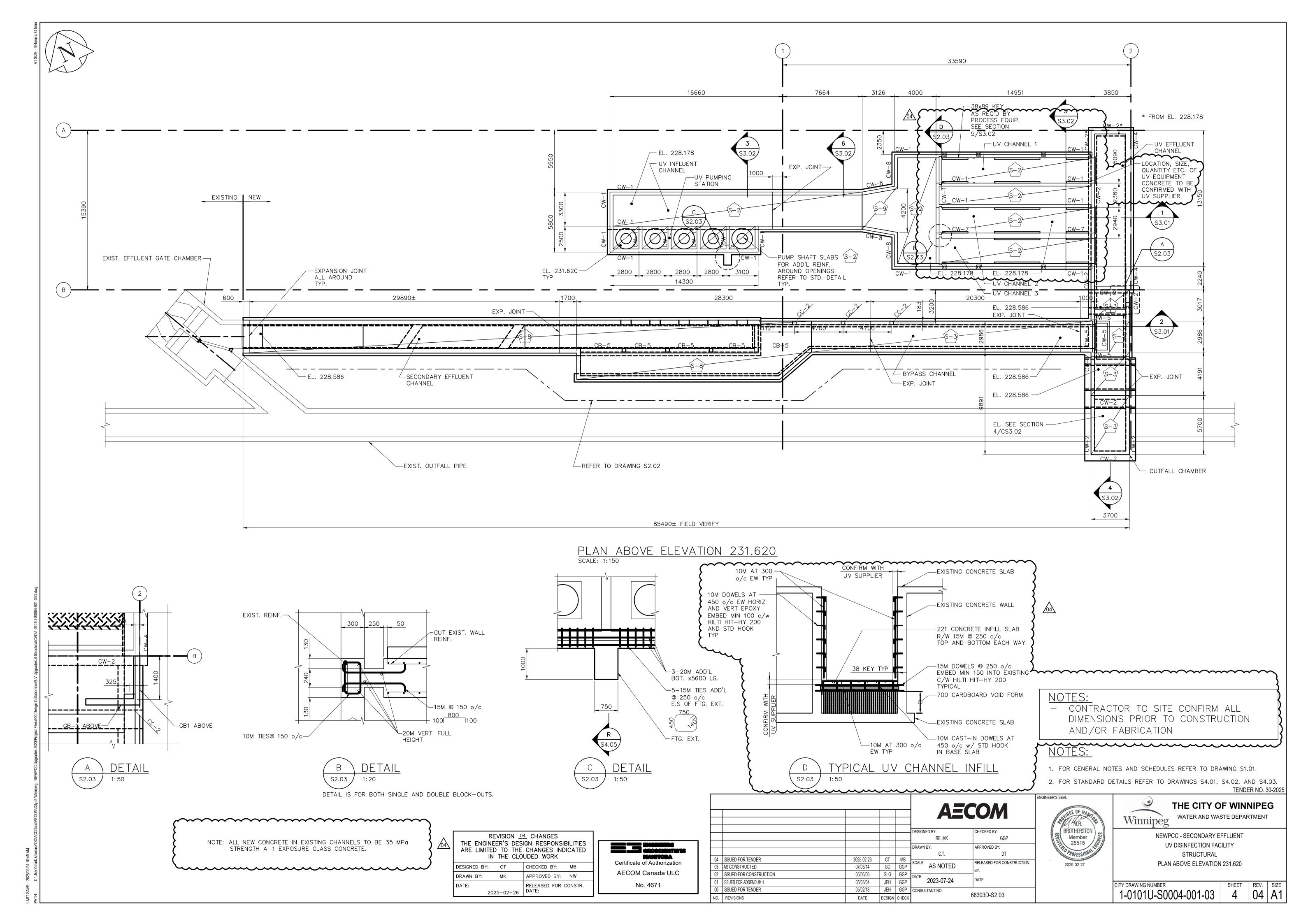
THE CITY OF WINNIPEG OF MANAGEMENT OF MANAGEMENT OF MANAGEMENT OF THE STORY OF WATER AND WASTE DEPARTMENT Winnipeg

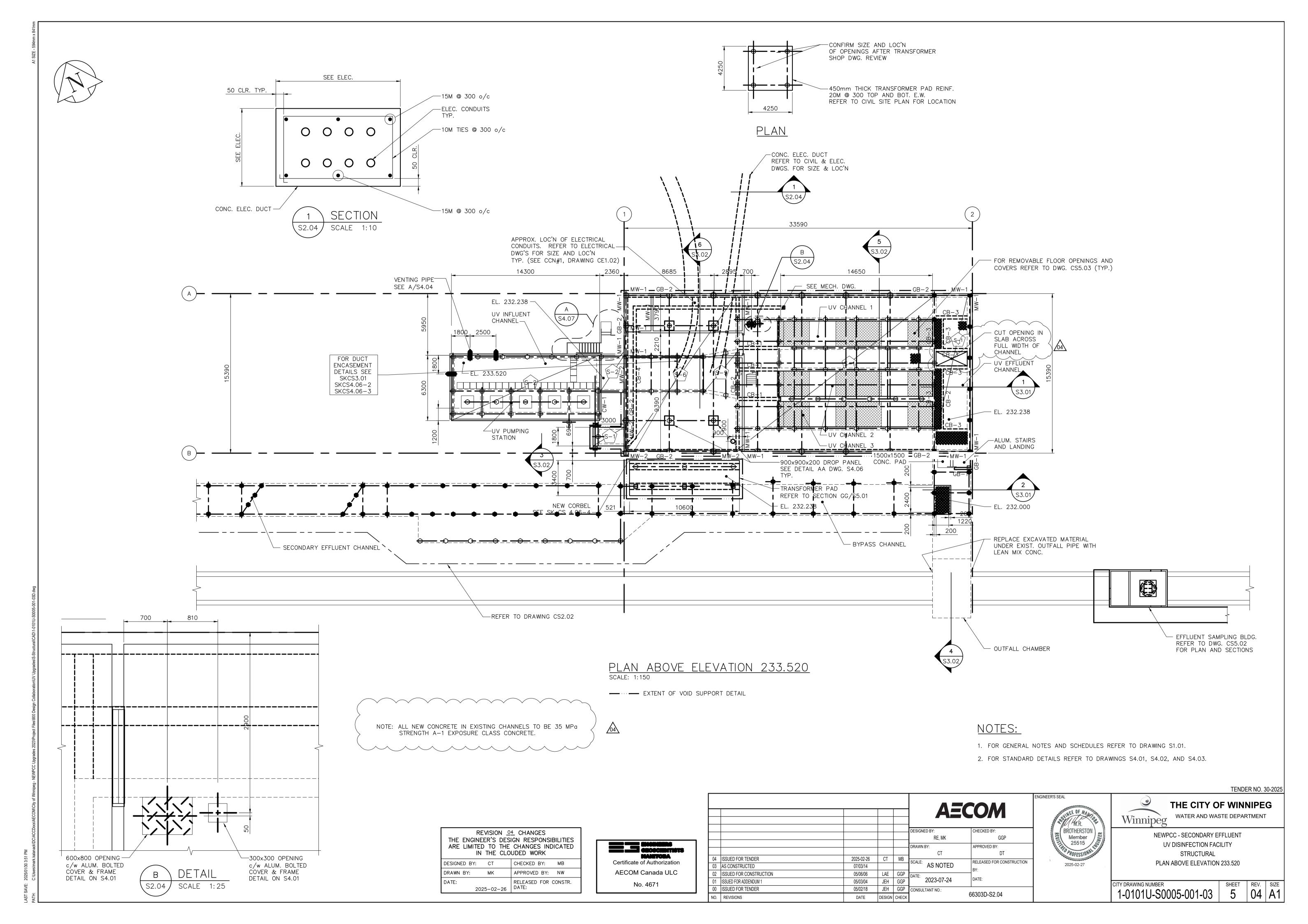
NORTH END SEWAGE TREATMENT PLANT NEWPCC UV UPGRADES STRUCTURAL NEW MONORAIL/ SUPPORT STRUCTURE ENLARGED PLAN, SECTION AND DETAIL

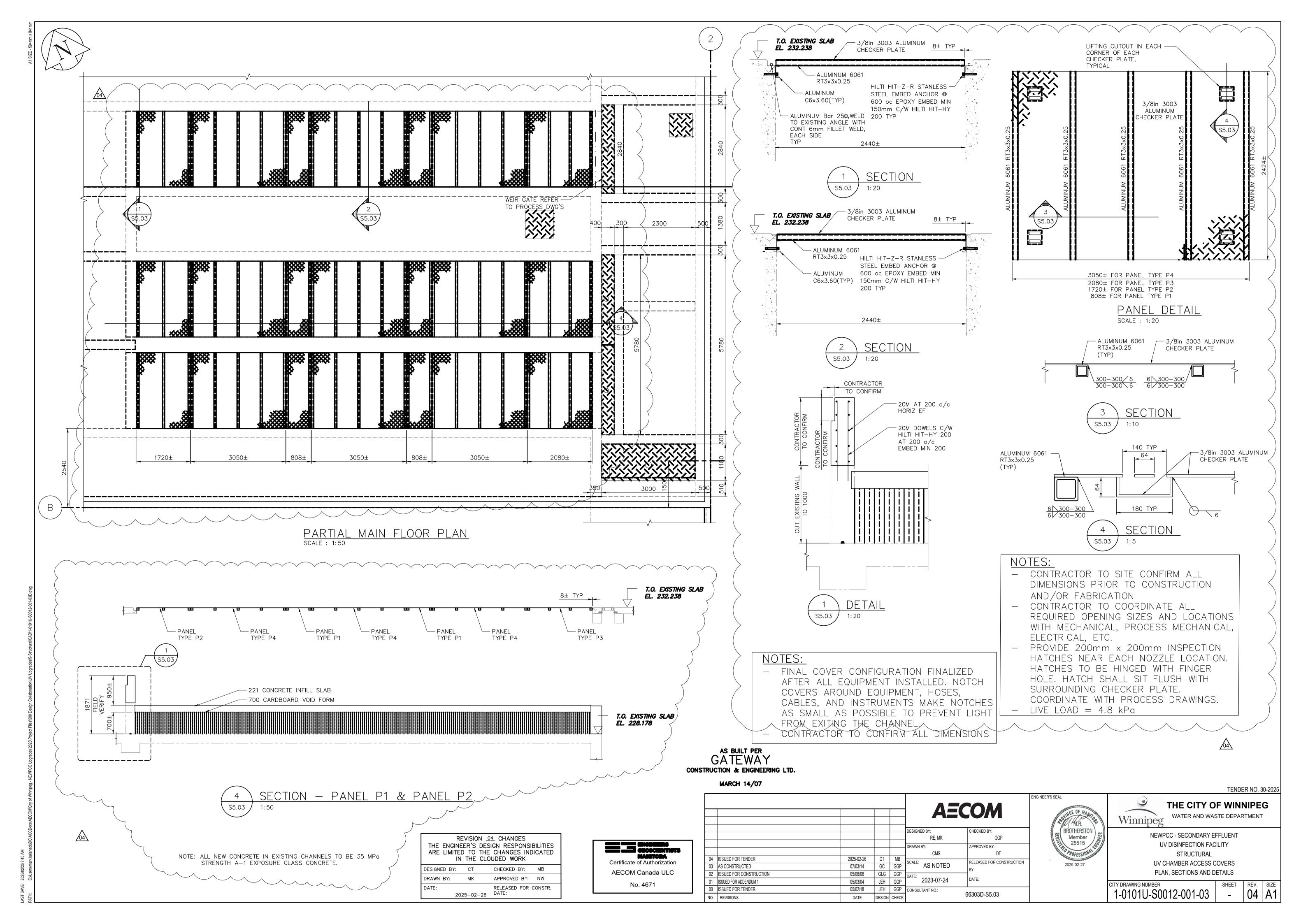
TENDER NO. 30-2025

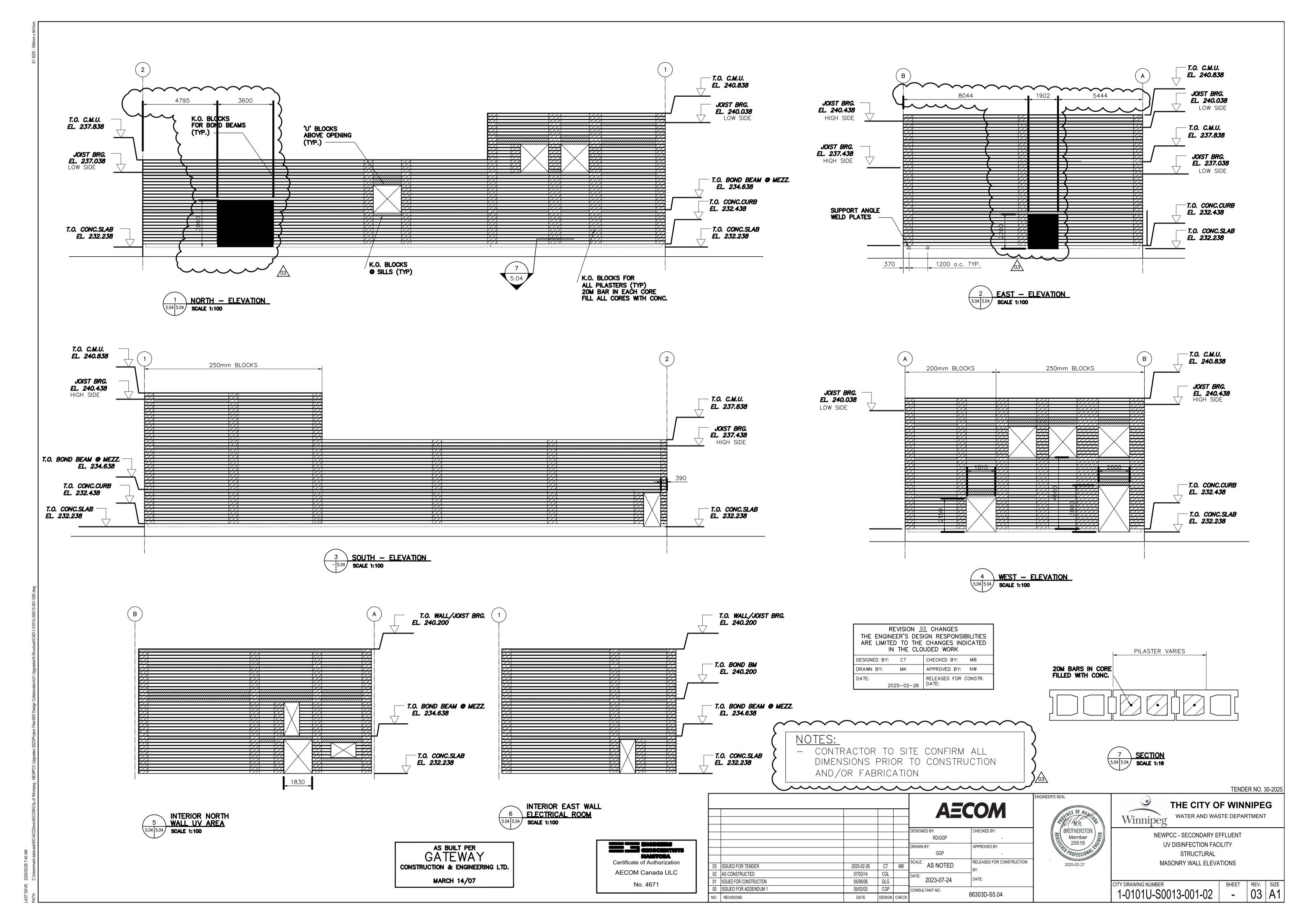
SHEET REV. SIZE 00 A1 CITY DRAWING NUMBER

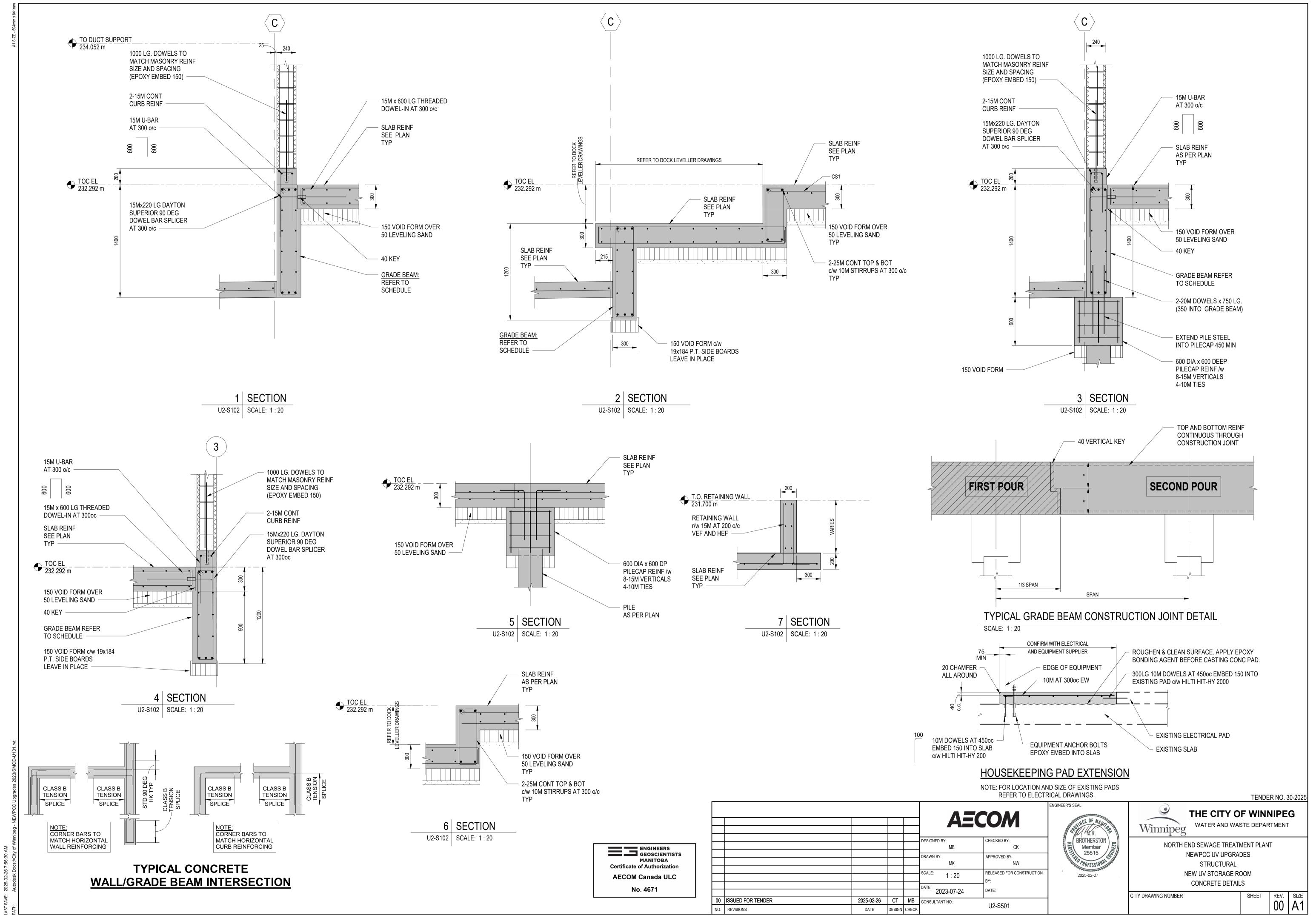


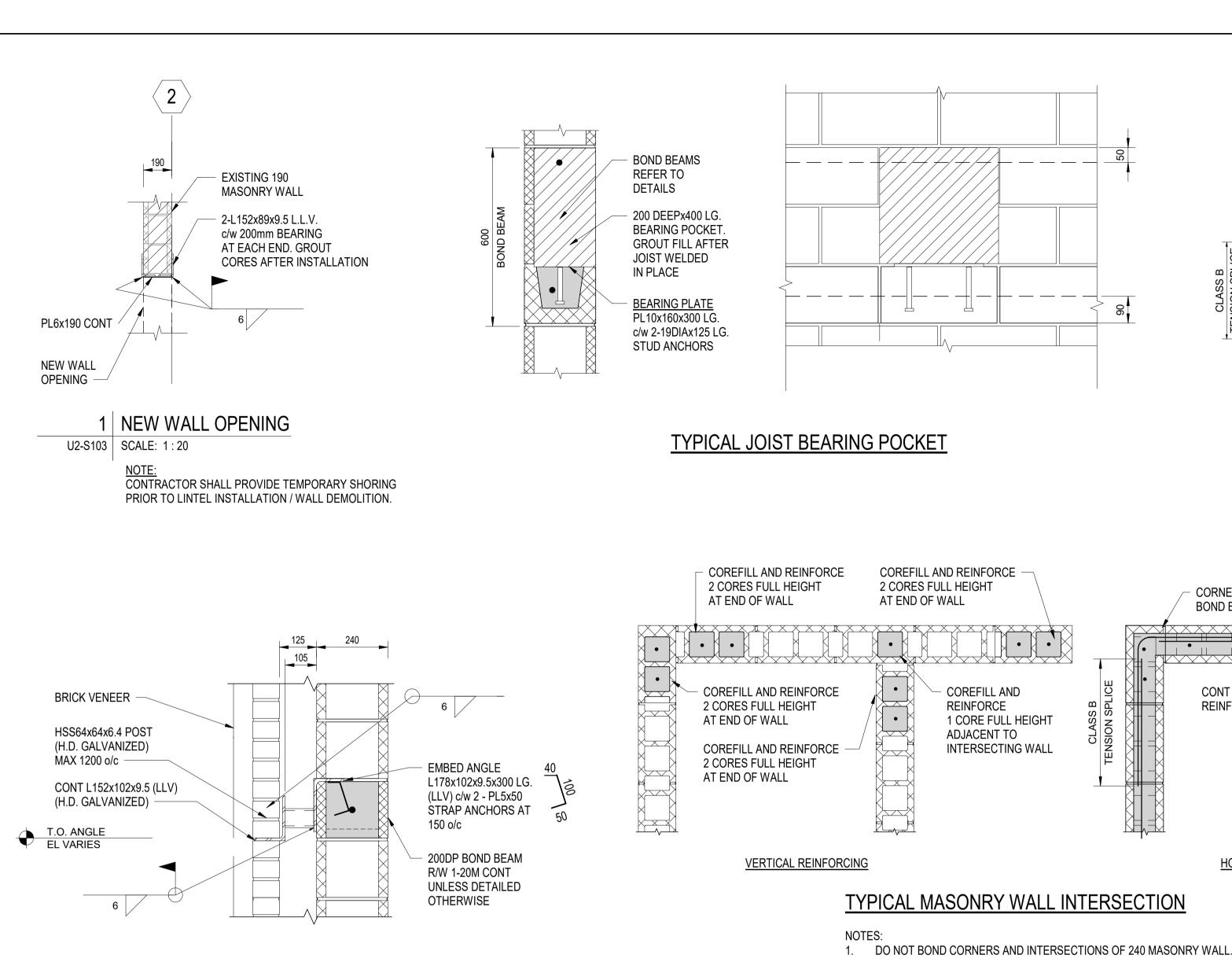


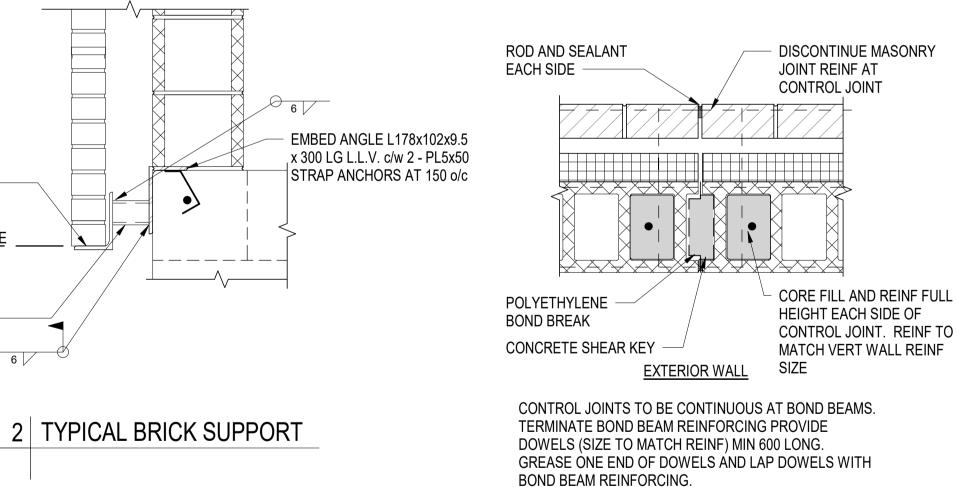




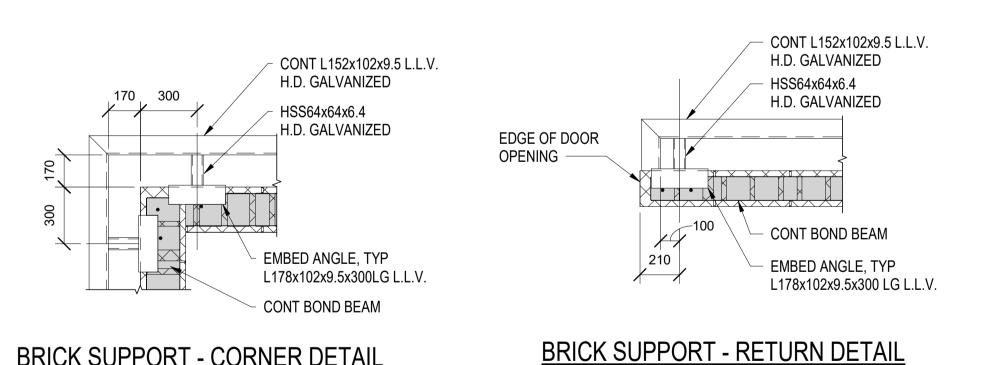








MASONRY WALL CONTROL JOINT



TOP OF MASONRY WALL REINFORCING DETAIL

CLASS B

TENSION SPLICE

VERT WALL REINFORCING

TO BE CONT THRU BOND BEAM.

DOWELS TO TOP BOND BEAM.

LAP WITH CLASS B TENSION SPLICE.

REFER TO TOP OF WALL DETAIL FOR

CORNER BARS TO MATCH

BOND BEAM REINFORCING

CONT BOND BEAM

HORIZONTAL REINFORCING

REINFORCING

MASONRY CONTROL JOINT AT OPENINGS

CONT BOND BEAM

VERT WALL REINF AS

DOWELS TO MATCH

VERT WALL REINF

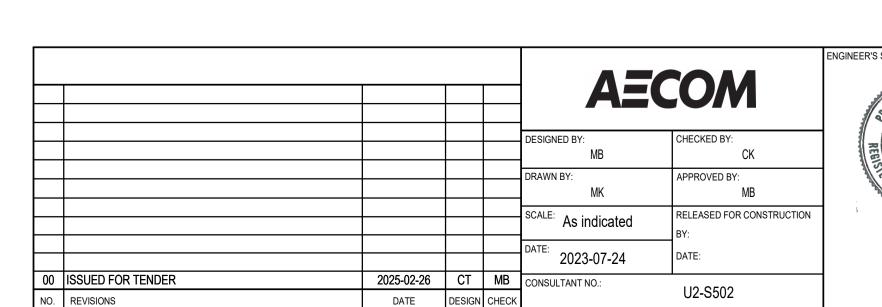
SIZE AND SPACING

PER PLAN

240 MASONRY LINTEL SCHEDULE							
OPENING LINTEL SIZE REINFORCING							
WIDTH	WIDTH	HEIGHT	TOP	воттом			
0 - 800	240	400	-	1-20M			
800 - 1200	240	400	1-15M	1-20M			
1200 - 2000	190	600	1-15M	1-25M			

1. LINTELS TO BEAR MINIMUM 200 BEYOND EACH SIDE OF OPENING

2. WHERE REINFORCING IS REQUIRED TOP AND BOTTOM, PROVIDE 10M STIRRUPS AT 200 o/c. FOR FULL LENGTH OF LINTEL



Winnipeg A REMARKE BROTHERSTON Member 25515

TENDER NO. 30-2025 THE CITY OF WINNIPEG

CORE FILL AND REINF FULL HEIGHT 2 CORES EACH SIDE OF OPENING

CONTROL JOINT TO TOP OF WALL REFER TO PLAN DETAIL OF JOINT

ON THIS DWG

LINTEL OVER OPENING REFER TO SCHEDULE

POLYETHYLENE BOND

BREAK UNDER LINTEL

BEARING AREA

- CONTROL JOINT REINFORCING

- CONTROL JOINT TO

BOTTOM OF WALL

REINF TO MATCH VERT WALL REINF SIZE

NORTH END SEWAGE TREATMENT PLANT NEWPCC UV UPGRADES STRUCTURAL NEW UV STORAGE ROOM

WATER AND WASTE DEPARTMENT

MASONRY DETAILS CITY DRAWING NUMBER SHEET REV. SIZE

00 A1

CONT

L152x102x9.5 L.L.V.

(H.D. GALVANIZED)

HSS64x64x6.4

MAX 1200 o/c

(H.D. GALVANIZED)

T.O. ANGLE

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BRICK SUPPORT - CORNER DETAIL

2025-02-27

