

ADDENDUM 3 BID OPPORTUNITY 171-2006

WINNIPEG WATER TREATMENT PROGRAM - SUPPLY AND INSTALLATION OF WATER TREATMENT PLANT PRECAST ROOF PANELS

URGENT

**PLEASE FORWARD THIS DOCUMENT TO
WHOEVER IS IN POSSESSION OF THE BID
OPPORTUNITY**

ISSUED: December 19, 2006
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**THIS ADDENDUM SHALL BE INCORPORATED
INTO THE BID OPPORTUNITY AND SHALL
FORM A PART OF THE CONTRACT
DOCUMENTS**

Template Version: A20050506

Please note the following and attached changes, corrections, additions, deletions, information and/or instructions in connection with the Bid Opportunity, and be governed accordingly. Failure to acknowledge receipt of this Addendum in Paragraph 10 of Form A: Bid may render your Bid non-responsive.

PART A – BID SUBMISSION

Replace: 171-2006_Addendum_2-Bid_Submission with 171-2006_Addendum_3-Bid_Submission. Form B(R1) has been replaced by Form B(R2) and Form G2(R2) has been replaced by Form G2(R3).

PART B – BIDDING PROCEDURES

Revise: B2.1 to read: The Submission Deadline is 12:00 noon Winnipeg time, January 24, 2007.

PART E – SPECIFICATIONS

Section 03412 (R1)

- Revise: 1.2.2 to read: Design all members and connections to safely support their own weight, superimposed loads and wind loads shown on Drawings, and all other forces and loads to which the structural members may be subjected. Special double-tee stem design may be required for large concentrated loads. Distribution of these loads and method of transfer into the stems will be reviewed by the Contract Administrator at the shop drawing stage. All precast concrete members shall act as roof in-plane diaphragms, typically bounded by cast-in-place perimeter beams with upstands or similar beams. Design in accordance with CSA A23.3-04, Clause 16.5.2.3. Spacing of connectors shall be determined by the precast concrete Manufacturer.
- Add: 1.2.9: Provide blockouts larger than 125 mm in size or diameter for mechanical and electrical installations in the double-tee decks at locations coordinated by the Contract Administrator.
- Add: 1.2.10: Openings smaller than 125 mm in diameter may be cored by others on site as coordinated by the Contract Administrator. These openings may only be located between the stems of double-tees in the uniform thickness portions of the deck.
- Revise: 2.1.9 to read: Provide 20 mm diameter holes at 1200 mm centre-to-centre through every stem on double-tee panels. Locate holes at approximately mid-height of stem. Holes may be formed with plastic sleeves to remain in the concrete.

Revise: 2.2.1 to read: Provide steel trowel finish for the double-tee top slab. Finish of formed surfaces shall conform to the requirements of CSA A23.4-05, Commercial Grade.

Revise: 2.5.1 to read: Connections, Supporting Devices: Flange connectors and panel end connectors of double-tee panels, and those connectors to be embedded in the cast-in-place beams opposite to precast panel edges, shall be Type 304 stainless steel. In addition, connector anchors for the embedded items in areas WF, WP and WR shall be Type 304 stainless steel. All other anchors, connectors and supporting devices shall be Type W Grade 300 steel, conforming to CSA G40.21-04, all galvanized to CAN/CSA G164-M92 (R2003), 600 g/m² min. zinc coating after fabrication. Provide embedded steel shoe plates at double-tee stem bearing ends. Refer also to Section 2.5.5(b) below.

Section 03414

Revise: 1.2.2 to read: Design all members and connections to safely support their own weight, superimposed loads and wind loads shown on Drawings, and all other forces and loads to which the structural members may be subjected. Special double-tee stem design may be required for large concentrated loads. Distribution of these loads and method of transfer into the stems will be reviewed by the Contract Administrator at the shop drawing stage. All precast concrete members shall act as roof in-plane diaphragms, typically bounded by cast-in-place perimeter beams with upstands or similar beams. Design in accordance with CSA A23.3-04, Clause 16.5.2.3. Spacing of connectors shall be determined by the precast concrete Manufacturer.

Revise: 1.2.9 to read: Concrete topping shall be 50 mm thick at the apex of the precast member camber. Design members such that the camber for the composite section due to all dead loads is minimized.

Add: 1.2.10: Provide blockouts larger than 125 mm in size or diameter for mechanical and electrical installations in the double-tee decks at locations coordinated by the Contract Administrator.

Add: 1.2.11: Openings smaller than 125 mm in diameter may be cored by others on site as coordinated by the Contract Administrator. These openings may only be located between the stems of double-tees in the uniform thickness portions of the deck.

Revise: 2.1.9 to read: Provide 20 mm diameter holes at 1200 mm centre-to-centre through every stem on double-tee panels. Locate holes at approximately mid-height of stem. Holes may be formed with plastic sleeves to remain in the concrete.

Revise: 2.2.1 to read: Provide raked finish for the double-tee top slab. Finish of formed surfaces shall conform to the requirements of CSA A23.4-05, Commercial Grade.

Revise: 2.5.1 to read: Connections, Supporting Devices: Flange connectors and panel end connectors of double-tee panels, and those connectors to be embedded in the cast-in-place beams opposite to precast panel edges, shall be Type 304 stainless steel. In addition, connector anchors for the embedded items in areas WF, WP and WR shall be Type 304 stainless steel. All other anchors, connectors and supporting devices shall be Type W Grade 300 steel, conforming to CSA G40.21-04, all galvanized to CAN/CSA G164-M92 (R2003), 600 g/m² min. zinc coating after fabrication. Provide embedded steel shoe plates at double-tee stem bearing ends. Refer also to Section 2.5.5(b) below.

Revise: 2.8.4 to read: Concrete topping shall be reinforced with welded steel wire fabric specified in 2.4.3. Size of mesh 152 x 152 MW(a) x MW(a). Wire cross-sectional area (a) shall be determined by the composite design requirements.

Add: 2.8.5: Adherence of bonded topping to the substrate shall be tested in accordance with Clauses 7.6.4.2.3 and 7.6.4.2.4 of CSA Standard A23.1-04. Topping not conforming to the requirements and delaminated topping shall be removed and replaced by the Contractor at the Contractor's cost.

- Revise: 3.1.13 to read: Fill all joints and grout keys between hollow core slabs to a minimum depth of 50 mm with 1:3 mixture of cement and sand, trowel smooth.
- Revise: 3.3.4 to read: Keep the substrate continuously moist prior to applying the bonding system. The topping concrete shall be bonded to the precast panels in accordance with CSA A23.1-04, Clause 7.6.4.2.2. Bonding procedure will be reviewed by the Contract Administrator at the submittal stage.
- Add: 3.3.7: Install topping such that it will not enter the joints between the double-tees.

The following Sections have been added and form part of this Addendum:

- 07515 – SBS Modified Bituminous Roofing Over Concrete Topping
07517 – SBS Modified Bituminous Roofing

DRAWINGS

The following Drawings have been added and form part of this Addendum:

<u>Consultant Drawing No.</u>	<u>City Drawing No.</u>	<u>Title</u>
WB-B0145	1-0601B-A-B0145-001-00D	ARCHITECTURAL – ROOF PLAN
WB-B0401	1-0601B-A-B0401-001-00D	ARCHITECTURAL – ROOFING AND PARAPET DETAILS WITH CONCRETE TOPPING
WB-B0402	1-0601B-A-B0402-001-00D	ARCHITECTURAL – ROOFING AND PARAPET DETAILS WITHOUT CONCRETE TOPPING
WB-B0403	1-0601B-A-B0403-001-00D	ARCHITECTURAL – ROOFING DETAIL AT STEEL PENTHOUSE